

# Stormwater Management & Infrastructure Design

Presented by:

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Transportation & Public Works Department

Stormwater Management & Development Services Division

November 14, 2018



# Agenda

- Intro
- Announcements, Reminders, & Process
- Design Standards, Common Errors, & Examples



# Announcements & Reminders

- SWFMA Fee
- Submission of Floodplain Development Permits
- Final SWMP submitted 7 days before IPRC Pre-sub
- TRWD and Water Quality
- Moving FDP & SWMP to Accela in 2019
- Stormwater Criteria Manual Update in 2019
- NOAA Atlas



### SWFMA Fee

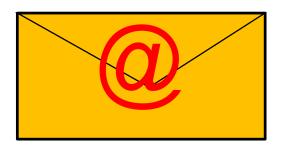
- \$635 paid at time of hard copy submission
- SDS continues to provide review of electronic exhibits during SWMP review
- A procedure and guide is available at:
   <a href="https://projectpoint.buzzsaw.com/fortworthgov/SDS/3%20-%20Resources\_Public/Stormwater%20Facilities%20Maintenance%20Agreement%20%28SWFMA%29/Stormwater%20Facility%20Maintenance%20Agreement%20Procedure.pdf?public</a>
- Questions? Email us at <u>SWFMA@fortworthtexas.gov</u>



# Original Submission Process

SWMP submissions





Digital documents to SDS via email or upload.

FDP submissions





Hard copy documents hand delivered to staff



# New Submission Process (effective 8/20)

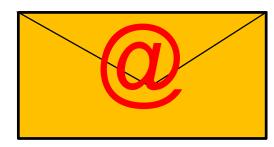
SWMP submissions





FDP initial submissions





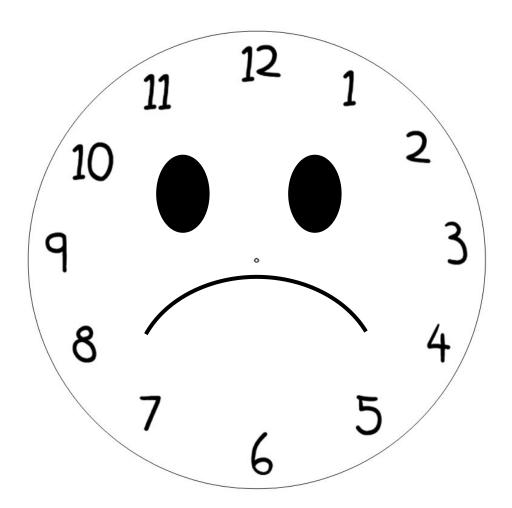
Digital documents to SDS via email or upload. \*

SDS@fortworthtexas.gov

<sup>\*</sup> Final hard copy report for record filing or for outside agency review.

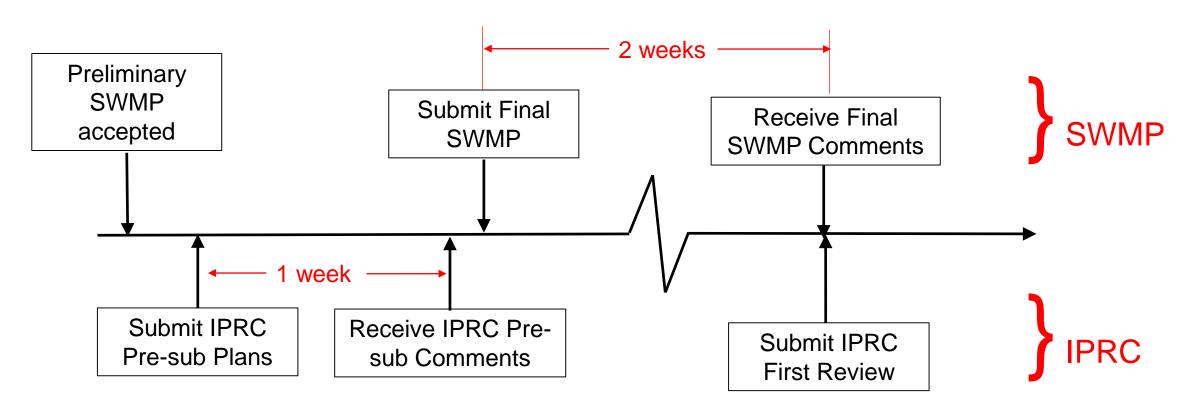






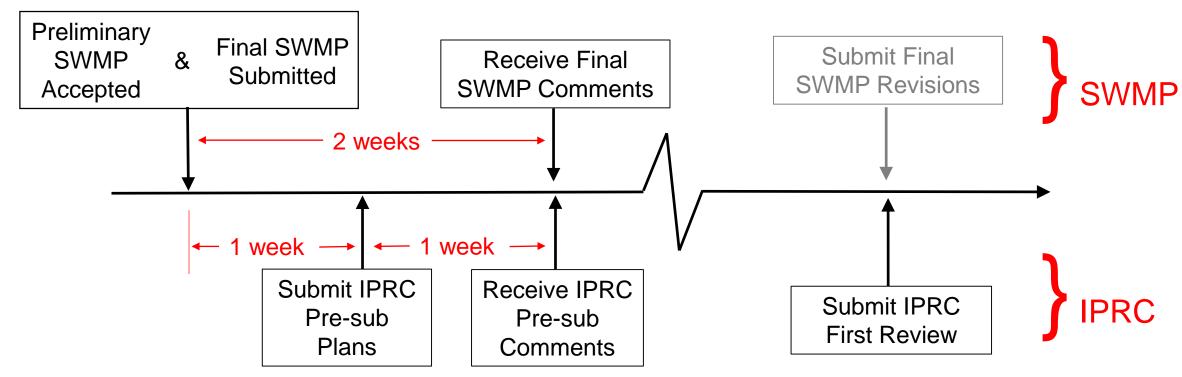


# Original Process / Timeline





### New Process / Timeline





### **SWMP & IPRC Process Links**

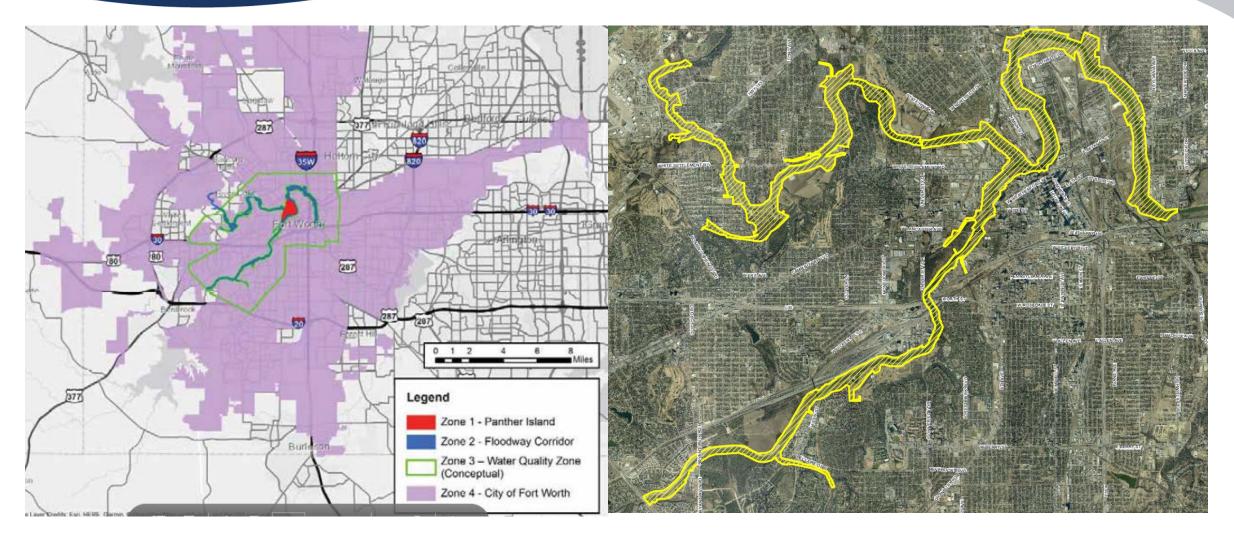
- Preliminary & Final SWMP IPRC Pre-sub
- Final SWMP acceptance Pre-con
- What is the difference between Final SWMP and IPRC review keep infrastructure review in IPRC



# Tarrant Regional Water District (TRWD)

- Newly published Water Quality guidelines: <u>http://www.trwd.com/water-supply/environmental/environmental-stewardship/water-quality/stormwater/</u>
- Green infrastructure, not grey infrastructure
- Success requires early site planning for green infrastructure
- Contacts:
  - Brenton Dunn <u>Brenton.Dunn@trwd.com</u> (primary contact)
  - Woody Frossard Woody.Frossard@trinityrivervision.org







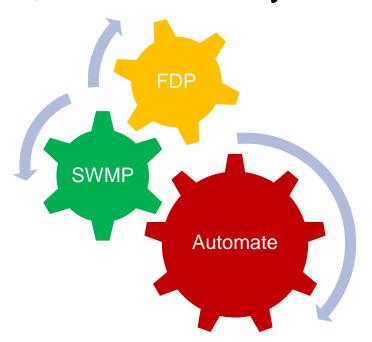
# General Process: Water Quality Review

- Submit conceptual green infrastructure plan for review
- Work with TRWD to reach consensus on concept plan
- Submit detailed design calculations signed and sealed by PE
- Work with TRWD to reach approval of design
- After permit is issued and construction is completed, submit a report signed and sealed by PE confirming that construction complied with design.



### SDS & Accela

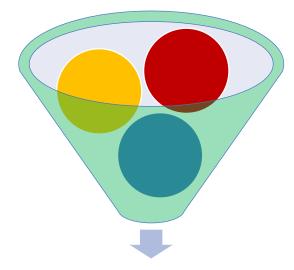
- Building on success of IPRC transition to Accela
- Transition FDP, SWMP, & Flood Study to Accela





# Stormwater Criteria Manual

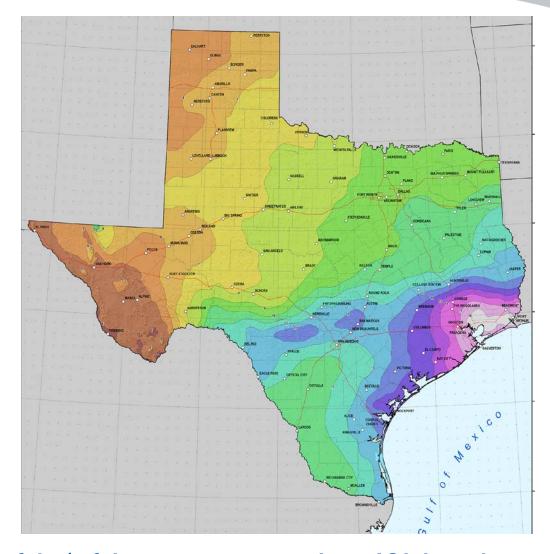
- Update planned for 2019
- Refinements & clarifications



- Contact:
  - Clair Davis <u>Clair.Davis@fortworthtexas.gov</u>
  - Stephen Nichols <u>Stephen.Nichols@fortworthtexas.gov</u>



# NOAA Atlas 14



Available at:

https://hdsc.nws.noaa.gov/hdsc/pfds/pfds\_map\_cont.html?bkmrk=tx



### iSWM Review Comment Assessment

- Purpose
  - Evaluated reviews to identify source or reason for review iterations and identify opportunity for program refinement
  - Reduce time and effort for development community (and city)
- Assessment performed by two firms JGR & PK
- Sample of 20 project reviews with 3 or more iterations
- Mix of preliminary and final iSWM plans, construction plans
- 14 categories of comments



### 14 Firms Represented in Sample of 20 Projects

- Frequent customers 9
- Occasional customers 3
- One time customers 2



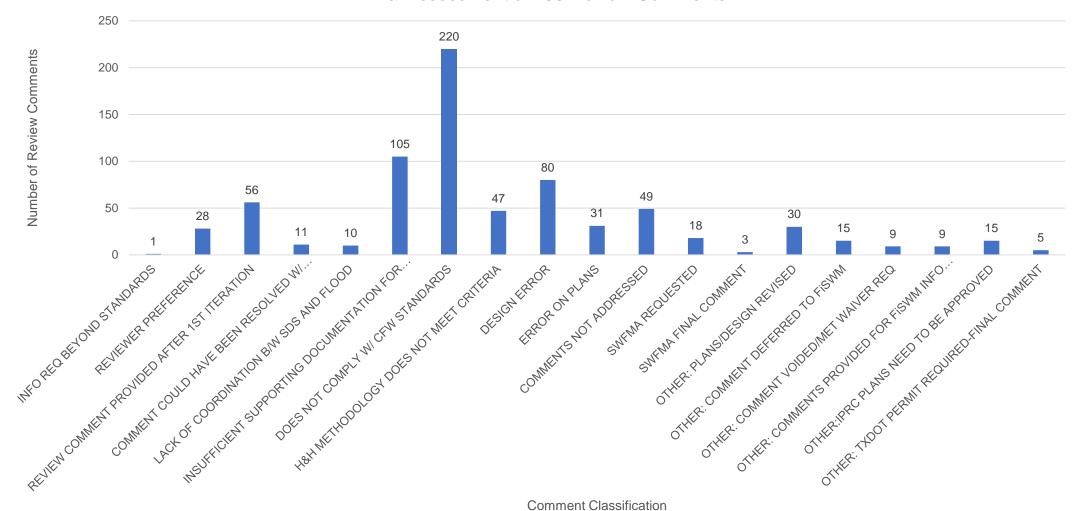
# Summary of Findings

Total of project samples affected by:

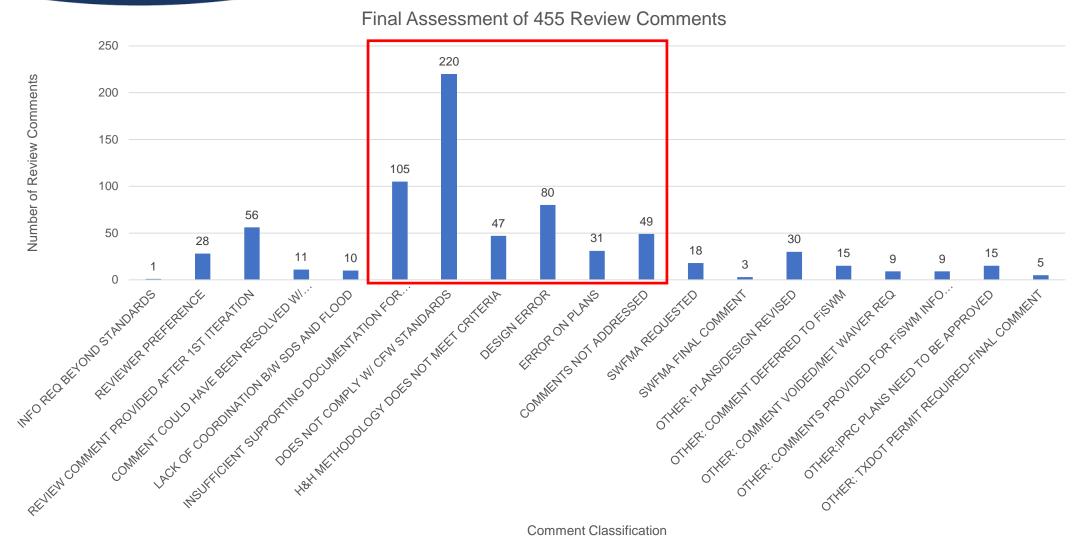
- Previous comments were not being addressed (100%)
- Submittals were missing data that was required in order to perform a review (94%)
- Submittals did not follow CFW criteria or show sufficient information to demonstrate compliance (59%)
- Design error/error on plans (59%)



#### Final Assessment of 455 Review Comments









# Summary of Findings (cont.)

- Does not comply with CFW standards (220)
- Insufficient supporting documentation for review (105)
- Design error conflicting information (80)
- Review comment provided after 1st iteration (56)
- Comments not addressed (49)
- H&H methodology does not meet criteria (47)
- Reviewer Preference (28)



# Design Standards & Common Errors

- Manhole Locations
- Lateral Slopes
- Lateral Connections & Inlets
- SFR Lot Grading
- Erosion Protection at Outfalls & Sediment Control
- Convey Runoff to Receiving Flowline
- Parallel RCP and Box Culverts
- CFW Standard Data Tabulations



### Manhole Locations

- Installed at upstream end and per stated Manual interval
- Locate correctly to avoid revising hydraulic tables
- Criteria Manual page 3-32:

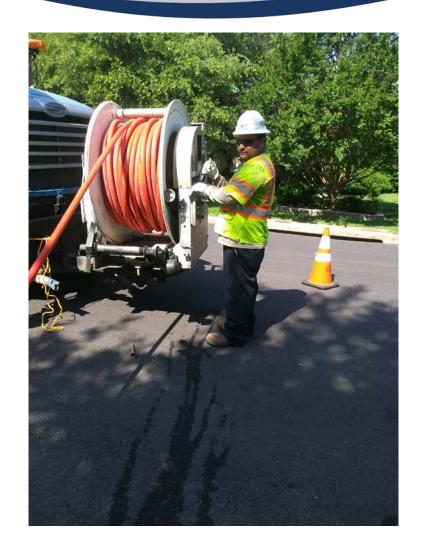
#### Manholes

Manholes shall be located at intervals not to exceed 550 feet for pipe 54 inches in diameter or smaller. For pipes sixty (60) inches in diameter and larger or equivalently sized boxes, the maximum interval is 800 feet. Manholes must be installed at the upstream end of a system and where a storm drain leaves the pavement, unless the outfall is within fifty (50) feet of the roadway and directly accessible. Manholes shall preferably be located at street intersections, sewer junctions, changes of grade and changes of alignment. When the storm drain is a concrete box instead of an RCP, four (4) foot diameter manhole risers may be installed instead of vaults to provide access. In all cases, steps or ranges shall be installed from the ground surface to the flowline of the pipe.







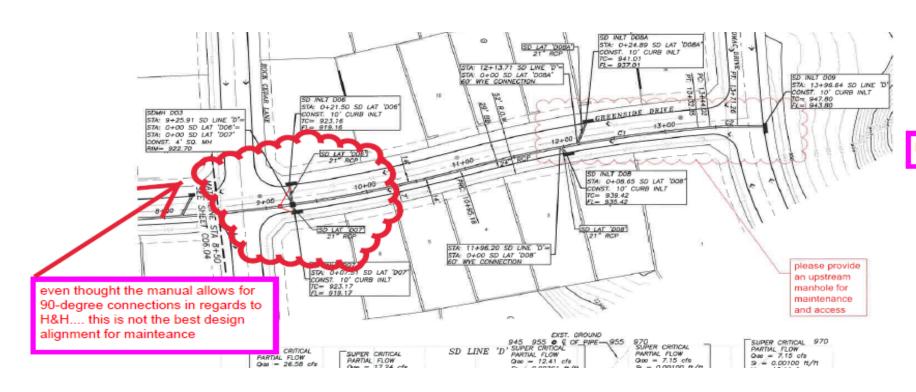








### Manhole Locations & Feedback from Field Ops



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CP #25 - HUTT-JOLLARS PREMARY COMMISS.

EM STAMPED WALCH RANCH SURMEY COMMISS.

A PAGINAE THAT BEARS H 47 E8'01 W. A D

A 6/8 INCH 180N ROD MITS CAP STAMPED W

SHEET NOTES: 1. REFER TO STORM DRAIN DETAIL S C10.05 & C10.09 TO C10.14.

per the manual this is acceptable

RE FROM BACK LINE OF THE RI E MOTED.

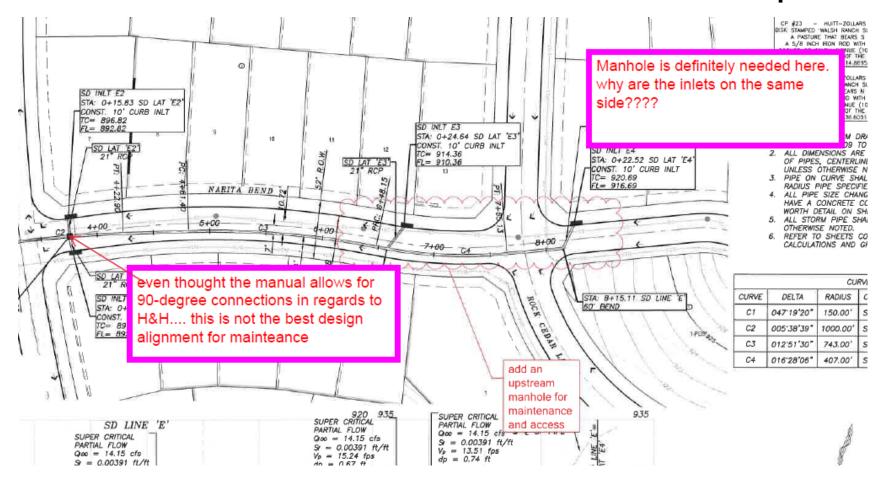
HAVE A CONCRETE COLLAR INSTAL
WORTH DETAIL ON SHEET CIO.23
ALL STORM FIRE SHALL BE CLASS
OTHERWISE NOTED.
REFER TO SHEETS COG.31 & COG.

CALCULATIONS AND GRADATION RE

		CURVE TABLE	
CURVE	DELTA	RADIUS	CH BEARING
C1	011'23'25"	1193.00*	S66'06'44'W
C2	005'12'12"	300.00"	S74'26'33'W

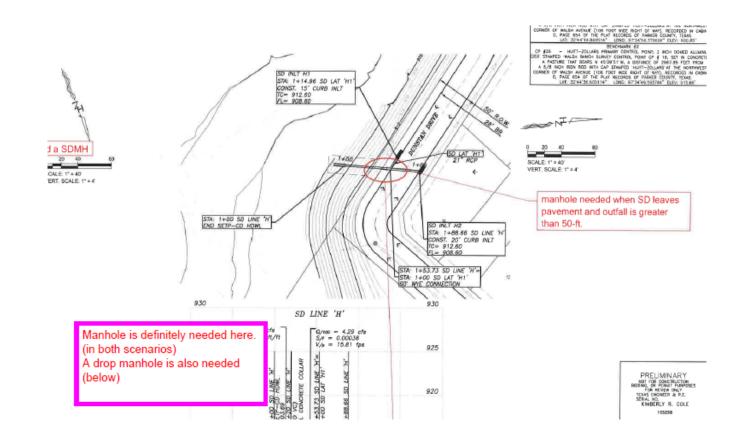


### Manhole Locations & Feedback from Field Ops





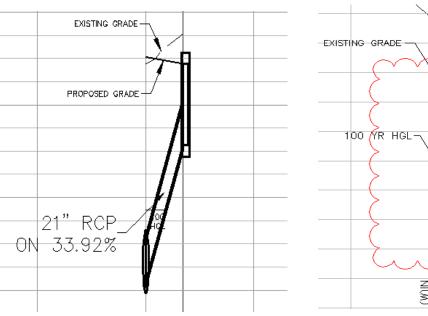
### Manhole Locations & Feedback from Field Ops

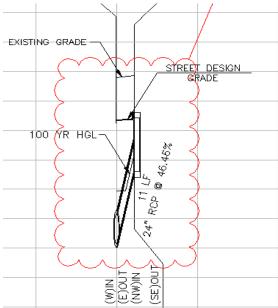




# Lateral Slopes

- Lateral Slopes
  - < 30% with a deeper inlet
  - Ease of access and constructability

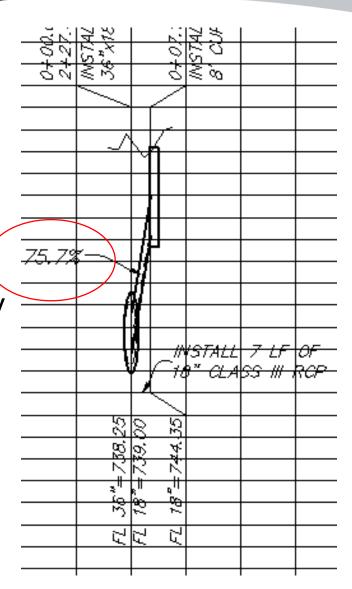






# Lateral Slopes

- Lateral Slopes
  - < 30% with a deeper inlet
  - Ease of access and constructability





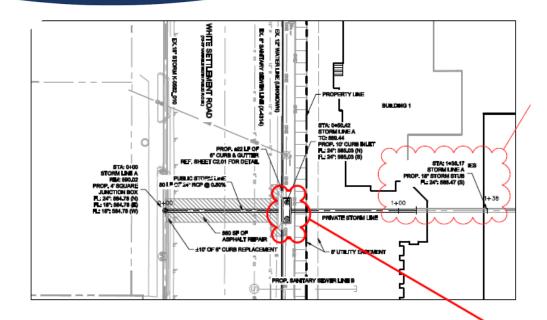
### **Lateral Connections & Inlets**

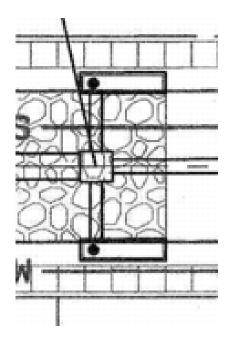
- Lateral connection plumbed to manhole opening
- Use a standard length inlet 10', 15', 20'
- Criteria Manual page 3-79

#### Inlets

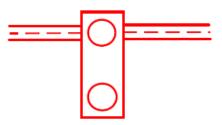
All curb inlets shall be five (5), ten (10), fifteen (15) or twenty (20) feet in length and shall have depressed openings. Recessed inlets shall be provided on arterial streets. Proposed inlet lengths greater than twenty (20) feet must be accepted by TPW. Care should be taken in locating inlets to allow for adequate driveway access between the inlet and the far property line. Standard inlet depth is 4.5 feet at the lateral line and 4.0 feet at the opposite end, with the bottom sloped to drain to the lateral line. Manhole steps shall be installed for any inlet over five (5) feet deep. Lateral lines shall be plumbed into the inlet at a manhole opening to expedite mechanical cleaning and inspection. A storm drain main may pass through an inlet if the system configuration allows and may substitute for manhole access.







Adjust proposed inlet and/or proposed storm drain line such that the SD lines enter and exit at manhole access locations (see exampe below).





# **Lateral Connections & Inlets**

- Use a standard length inlet 10', 15', 20'
  - Use of non-standard inlet length will result revisions to a standard length, revised hydraulic tables, and an additional review





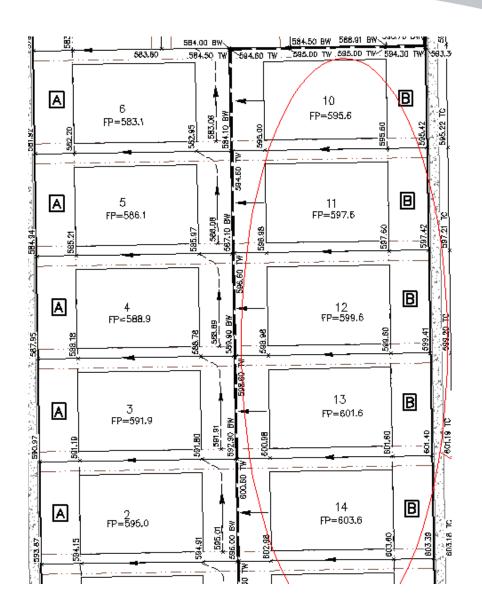
# SFR Lot Grading

- Type B/C to Type A lot with contiguous lot lines
- Type C to Type A lot with non-contiguous lot lines
  - Typically HUD Block Grading Type 3
- Refer April 2018 Newsletter



# SFR Lot Grading

 Type B/C to Type A with contiguous lot lines





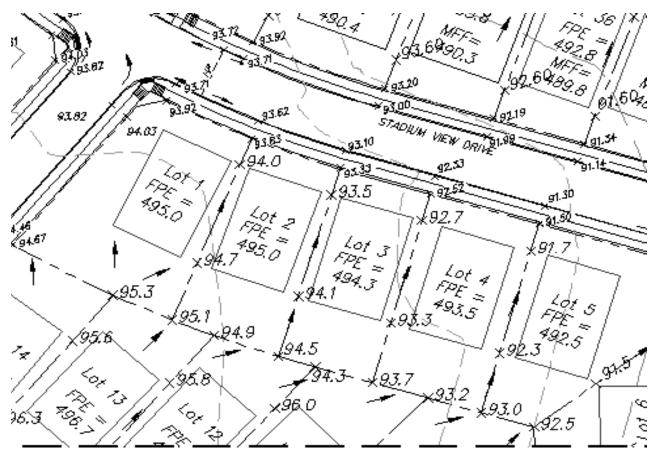
- Type C to Type A lot non-contiguous lot lines
  - Typically HUD Block Grading Type 3
  - Criteria Manual page 3-32

grading as shown in the FHA information is preferred. Type 3 and block 4 grading is allowed only if:

- a swale, flume or channel is constructed at the rear of the lot to intercept runoff; and
- runoff from 3 or more lots is collected and conveyed within an underground drainage system, swale, flume or channel contained within a dedicated easement.

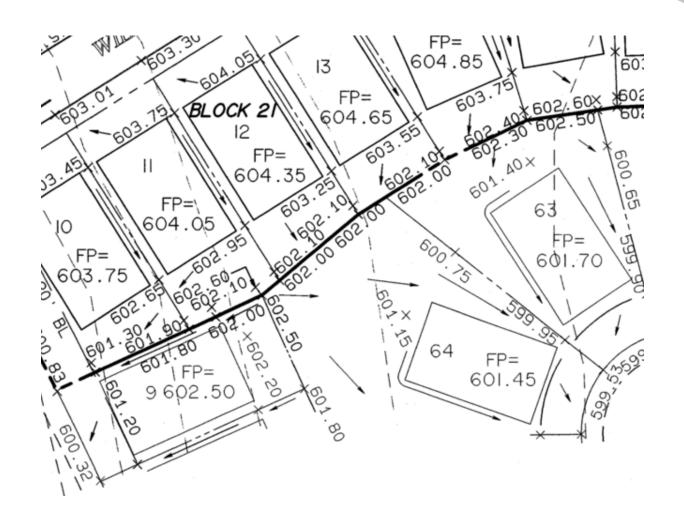


- Type C to Type A lot non-contiguous lot lines
- What about fence lines?





- Type C to Type A lot non-contiguous lot lines
- What about Lot 64?





#### • Reference April 2018 Newsletter

#### Single Family Residential (SFR) Lot Grading

The Stormwater Management staff worked with a group of development community representatives to clarify our current lot grading standards, improve consistency, and simplify the review process. Additionally, the current process is being reviewed to identify means for better documenting the builder's intention regarding lot grading as a part of the building permit process and some sort of certification from the builder regarding lot grading at the end of construction.

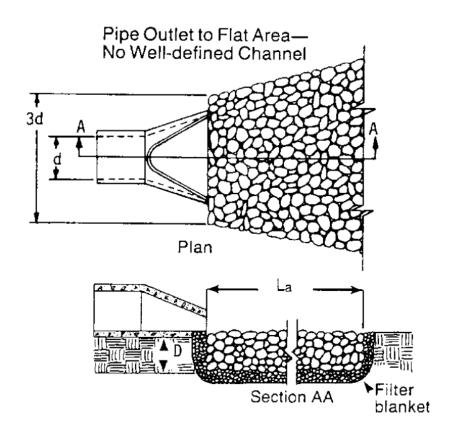
The outcome from the discussion was that Type 3 Block grading (type C lots draining to type A lots), the iSWM plan reviewer would allow up to a 5% slope in the swale in back yards (between rear building lines) without requiring a retaining wall or interceptor drainage system. The following conditions would also need to be met:

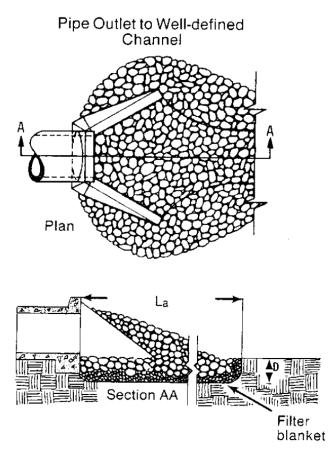
- 1. The swale should flatten and widen out as much as feasible before crossing lot lines in order to minimize erosion potential.
- 2. The design should make sure that flow is not lost to side neighbors and that flow through the swales is conveyed to the lot behind.
- 3. Swales shall be continuous and upstream side yard swales should align with downstream side yard swales.
- 4. The swale through back yard should never be less than 3 inches in depth.
- The separation between the side yard swale flowline and finished floor elevation should be a minimum of 9 inches at the upstream end (upstream lot) of the swale and minimum 12 inches at the downstream end.
- 6. The design engineer shall provide standard swale details for each subdivision at section which represent the varying swale conditions.



- Erosion and Sediment Protection at Outfalls
  - Properly Sized Hydraulics Manual page HA-213 to HA-215
  - Evaluate 1, 5, and 100 year events for energy dissipation
  - Use appropriate sediment BMP



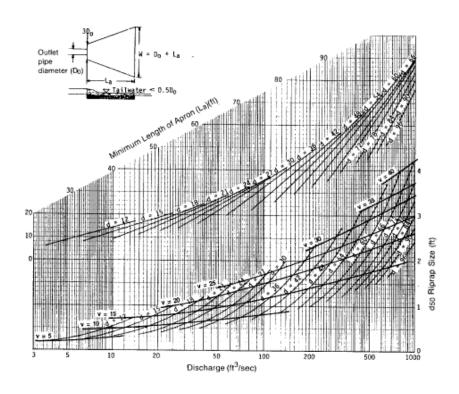




#### **Notes**

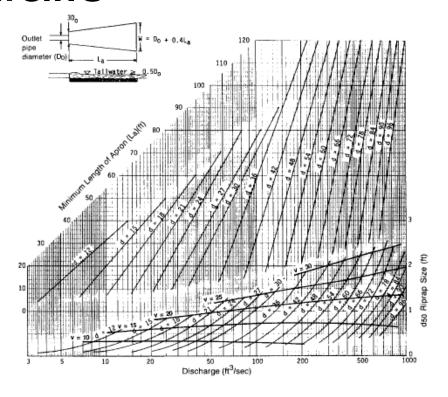
- L<sub>a</sub> is the length of the riprap apron.
- D = 1.5 times the maximum stone diameter but not less than 6".
- In a well-defined channel extend the apron up the channel banks to an elevation of 6" above the maximum tailwater depth or to the top of the bank, whichever is less.
- A filter blanket or filter fabric should be installed between the riprap and soil foundation.





Curves may not be extrapolated.

Figure 4.2 Design of Riprap Apron under Minimum Tailwater Conditions
(Source: USDA, SCS, 1975)



Curves may not be extrapolated.



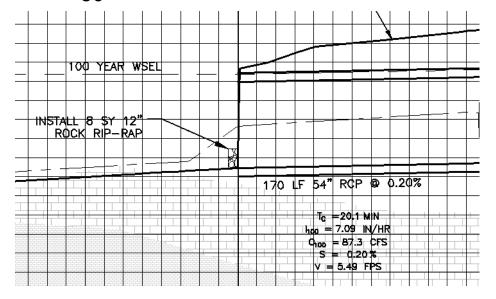
Properly Sized and Installed?

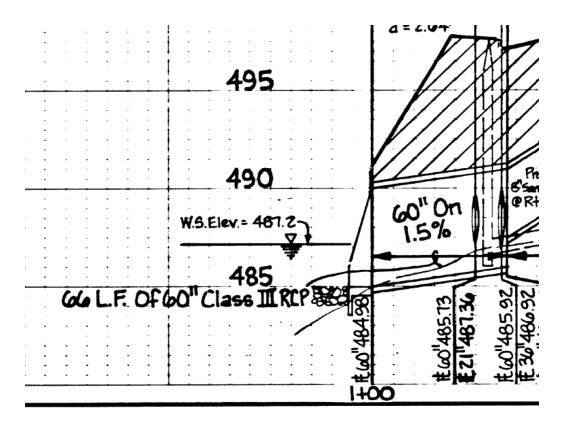






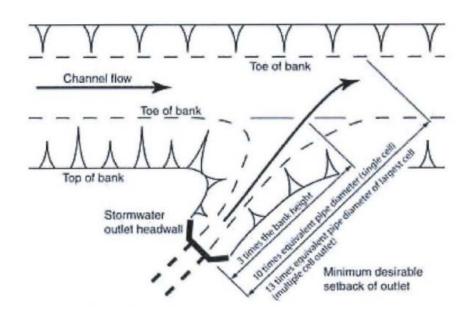
- Rule of Thumb
  - Apron length 4x-6x pipe diameter or width of box
  - D<sub>50</sub> of 12" and thickness of 18"







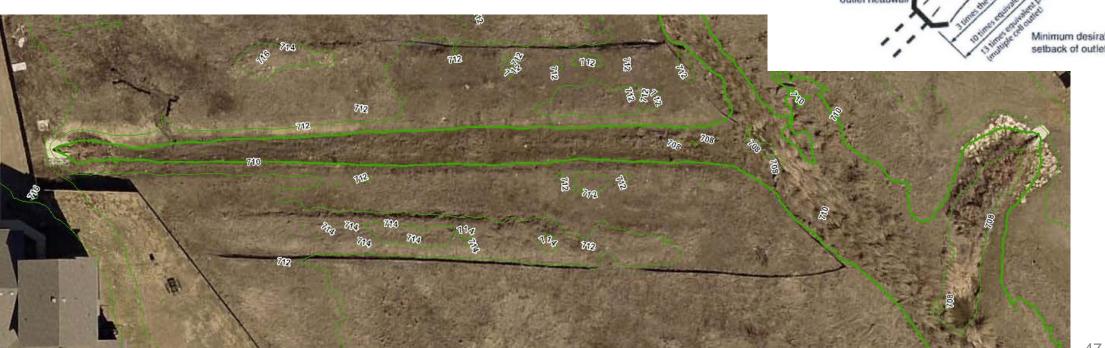
Consider adjacent banks







Consider adjacent banks





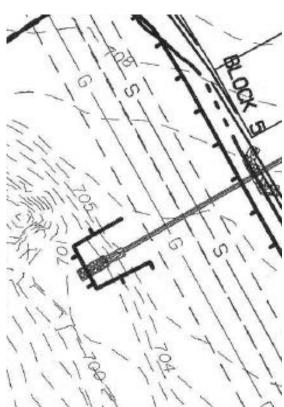
- Evaluate 1, 5, and 100 year events
- Tailwater at 100 year event may provide a buffer not available during low flow events



## Sediment Control at Outfalls

- Provide appropriate BMP
- Criteria Manual page 4-5
  - Linear controls (silt fence) not allowed in areas of concentrated flow















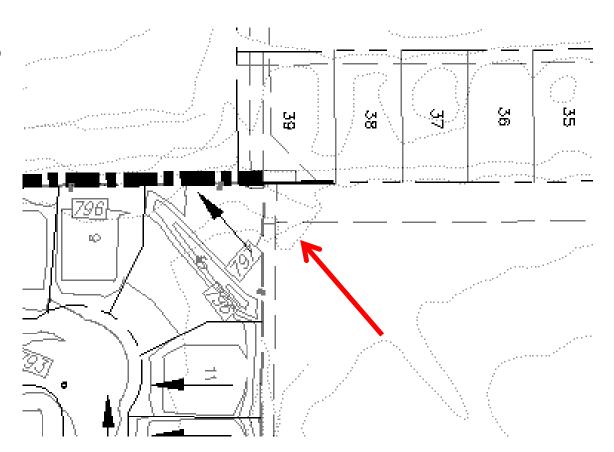






#### **Sediment Controls**

- Ensure existing flow paths are maintained
- Silt fence blocking existing path resulted in upstream flooding on Lot 39





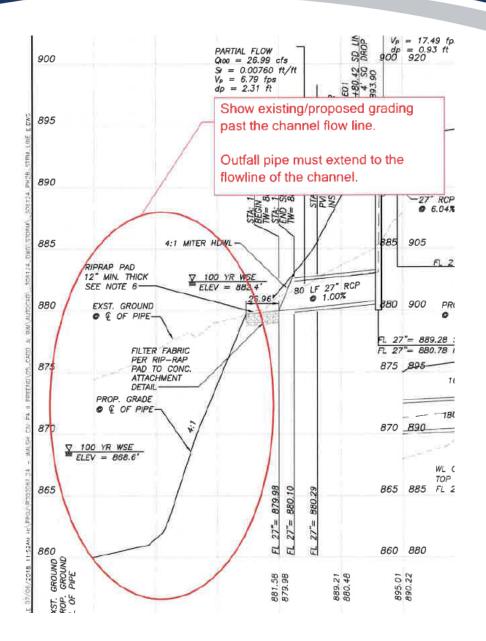
#### **Sediment Controls**

- Erosion Protection at Outfalls
  - Ensure existing flow paths are maintained
  - Resulting flooding due to silt fence across existing drainage way

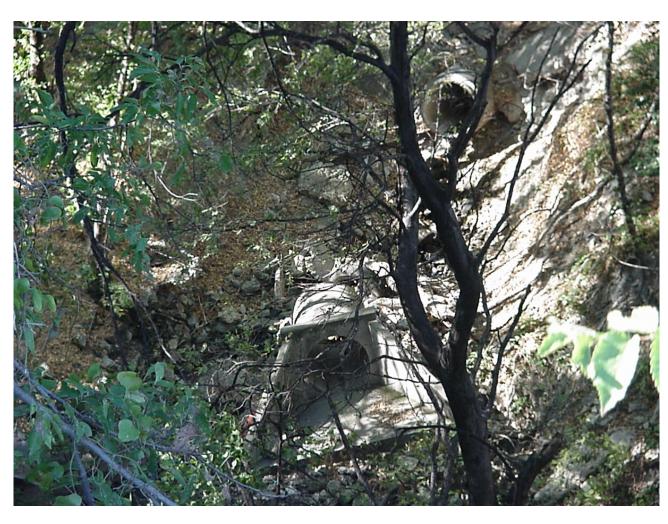




- Convey runoff to receiving flowline
  - Channel
  - Extend BMP to flowline
  - Return to sheet flow
  - Convey concentrated flow to concentrated receiving stream













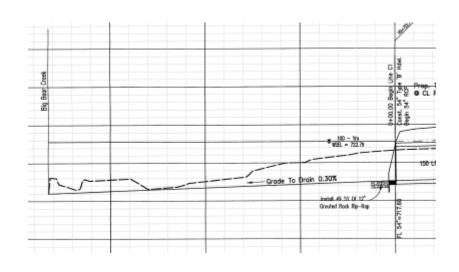


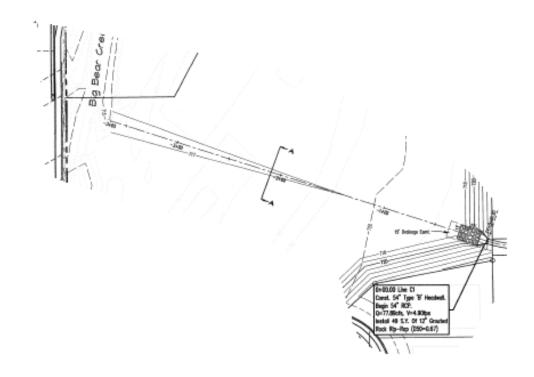






- Convey runoff to receiving flowline
  - Channel (Design)







- Convey runoff to receiving flowline
  - Channel (Practice)





 Channel not conveyed to a flowline, unintended channelization due to concentrated flow







 No designed conveyance to outfall, Stormwater has cut its own path



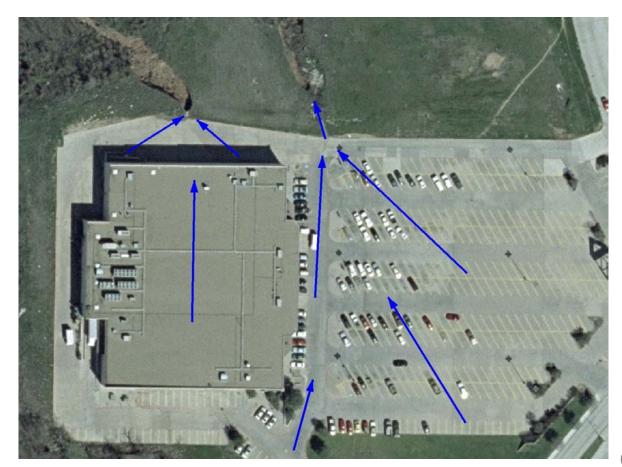


 No designed conveyance, Stormwater has cut its own path



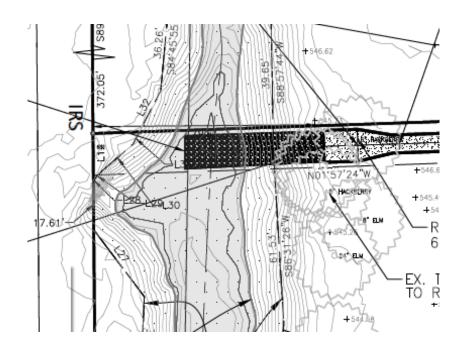


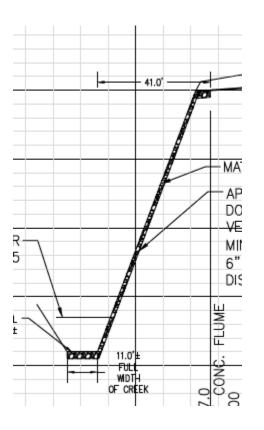
 No designed conveyance, Stormwater has cut its own path





Extend BMP to flowline



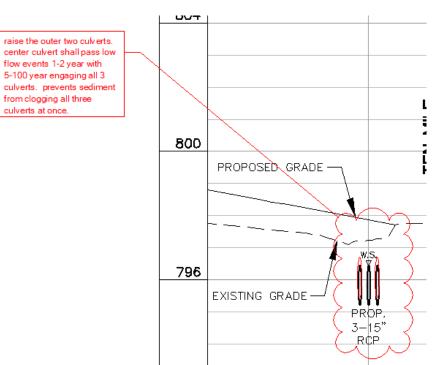




- Box Culverts and Parallel Runs
  - Criteria Manual page 3-55, low flow barrel at lower elevation
  - Low flow to carry 2% of 100 year event
    - For multiple barrel culverts the CFW requires the placement of one of the barrels at the flowline of the stream with the other barrels at a higher elevation to create a single flow path for lower flow and reduce sediment and debris accumulation. Where practical the low-flow portion of the low barrel(s) should convey 2% of the design 100-year discharge.



Example: driveway culvert requiring adjustment





Open bottom arch culverts at bankfull elevation





Baffles at bankfull allow flood passage





## Use CFW Standard Hydraulic Tables

- Refer the following figures in the manual
  - Figure 3.5 Curb Opening Inlets and Drop Inlets in Sump (page 3-25)
  - Figure 3.7 Computation Summary Sheet for Determining Capacity of Curb Opening Inlet on Grade (Depressed) (page 3-28)
  - Figure 3.10 Computation Sheet for Storm Drains (page 3-41)
- Ensure all data is completed

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1	2	3	4	5	6	7	8	9	10	11	12	13	14
EXISTING													
EX C-1-f	0.00	#VALUE®	#VALUEP	100-yr	0.00	5	0.00	0.00	0.00		0.00	0.00	Existing 5' Curb Inlet
EX H-1-a	0.00	#VALUER	#DIV/OP	100-yr	0.00	10	0.00	0.00	0.00		0.00	0.00	Existing 10' Curb Inlet
PROPOSED													
PRO C-1-f	0.00	#VALUE®	#DIV/OF	100-yr	0.00	10	0.00	0.00	0.00		0.00	0.00	Proposed 10′ Curb Inlet
PRO H-1-a	0.00	#VALUEP	#DIV/OP	100-yr	0.00	10	0.00	0.00	0.00		0.00	0.00	Proposed 10′ Curb Inlet

CLIMD INI ETC

